

**STREAMFLOW-GAGING STATIONS**

The U.S. Geological Survey operates streamflow-gaging stations to measure streamflow (table 1). These stations provide data required to describe runoff characteristics in the Westfield and Farmington River basins for land-use planning. Locations of these streamflow-gaging stations are shown in figure 5.

**HIGH FLOW**

A combination of large amounts of rainfall, the steep slope of the river channels, pronounced topographic relief, and low permeability of most of the surficial deposits in the Westfield and Farmington River basins has led to considerable and frequent flooding in the basins. The five greatest floods of record occurred in descending order of magnitude August 1955, September 1938, March 1936, November 1927, and December 1948. Flood profiles and high-water marks for the Westfield River and its major tributaries and the West Branch Farmington River have been published for the 1955 flood (Bogart, 1960) and the 1956 flood (Bogart, 1961). The 1955 flood (Bogart, 1960) was the greatest flood of record. Most of the great floods resulted from excessive rainfall runoff from snowmelt, or a combination of both. The steep slope of the rivers in the upper reaches contributes to high velocity floodflows. Channel slope profiles of the West Branch Farmington River, West Branch Westfield River, and Westfield River are shown in figure 6.

As can be seen on the maps on sheets 1 and 2 (fig. 2), very little stratified drift exists in the river basins except along the lower reaches of the Westfield and Little Rivers in Westfield. Because the infiltration of precipitation into the soil is low and there are few surface-water reservoirs in the basins, a very high percentage of the rainfall runs off. This occurs not only during extremely high flow events but also during 1955 flood flow but also during lower events the those of May-June 1984, as shown in figure 7.

It can be seen in figure 7 that the Westfield River near the City of Westfield has a lower percentage of runoff from rainfall than do the other two sites. This lower runoff percentage is attributed to relatively low channel slope and average basin slope, larger amounts of stratified drift, and large flood plain in the lower reach of the river near the City of Westfield. The flood control that exists in the upper reaches also reduces the runoff-rainfall ratio to some extent.

Considerable flood control is provided in the basins by multiple levees and reservoirs. A listing of these is shown in table 2. The West Branch Westfield River is the only totally uncontrolled large river in the Westfield River basin. The West Branch Farmington River is only partially controlled by Otis Reservoir on Fall River, in Massachusetts, but is more fully controlled just beyond the State line by Cobble Mountain Reservoir in Connecticut.

Probability curves of maximum annual peak discharges and highest average discharges for the indicated number of consecutive days for various streamflow-gaging stations are shown in figure 8. These flood-frequency curves are used for flood insurance and flood-control reservoir-capacity studies. The following examples demonstrate use of probability curves for the Valley Brook gaging station (fig. 9C).

Example No. 1: The probability that the highest average discharge for 15 consecutive days of a water year would exceed 100 ft<sup>3</sup>/sec (cubic feet per second) is 20 percent.

Example No. 2: There is a 2-percent chance during any water year that the discharge for the three consecutive days will exceed 700 ft<sup>3</sup>/sec.

Flood insurance studies showing the existence and severity of flood hazards have been published by the Federal Emergency Management Agency for most of the communities in the basin. These studies show the peak discharges, areas of flood inundation, and elevations of the 100-year and 500-year floods for major streams.

Flood information is essential to the economic design of flood-control and riverine structures. Equations for estimating the magnitude and frequency of floods in west-central Massachusetts have been developed by Wandell (1983). The equations apply to ungauged rivers where floodflows are ungauged and are within the extremes of the available data for drainage areas of 0.27 mi<sup>2</sup> to 145 mi<sup>2</sup>. They do not apply to sites where floodflows are significantly affected by diversion, urbanization, and regulation, or where the usable mainstem drainage area above the site. Additional explanation and examples of how to determine flood-frequency estimates are given in Wandell (1983).

The following equations provide estimates of annual peak discharges corresponding to the 0.5, 0.2, 0.1, 0.04, 0.02, and 0.01 annual exceedance probabilities.

Average standard error (percent)

Q <sub>0.5</sub> = 0.556A <sup>0.702</sup> Q <sub>1955<sup>0.181</sup></sub>	27
Q <sub>0.2</sub> = 0.556A <sup>0.702</sup> Q <sub>1955<sup>0.181</sup></sub>	28
Q <sub>0.1</sub> = 1.23A <sup>0.702</sup> Q <sub>1955<sup>0.181</sup></sub>	31
Q <sub>0.04</sub> = 1.31A <sup>0.702</sup> Q <sub>1955<sup>0.181</sup></sub>	36
Q <sub>0.02</sub> = 1.41A <sup>0.702</sup> Q <sub>1955<sup>0.181</sup></sub>	40
Q <sub>0.01</sub> = 1.51A <sup>0.702</sup> Q <sub>1955<sup>0.181</sup></sub>	45

In which:  
Q is the peak discharge, in cubic feet per second, for the specified exceedance probability (0.5, 0.2, 0.1, 0.04, 0.02, 0.01);  
A is the drainage area, in square miles;  
Q is the main-channel flow, in feet per mile, and  
E is the mean basin elevation, in feet.

Regression equations for determining the magnitude and frequency of urban floods were developed by Sauer and others (1983). These equations may be applied to the urbanized, ungauged areas of Westfield, West Springfield, and Agawam. The equations use flood-frequency estimates from equations 1 and 2 developed by Wandell (1983) in conjunction with topographic and climatic characteristics, land-use variables, and index of urbanization.

TABLE 1.—Streamflow-gaging stations  
[—, no regulation; wq, weaver miles; ft<sup>3</sup>/s, cubic feet per second]

Stream and location	Drainage area (mi <sup>2</sup> )	Period of record	Regulation structure and year completed	Maximum discharge before regulation (ft <sup>3</sup> /s)	Maximum discharge after regulation (ft <sup>3</sup> /s)	Average discharge before regulation (ft <sup>3</sup> /s)	Average discharge after regulation (ft <sup>3</sup> /s)	
Westfield River at Huntington	161	August 1909–September 1986	Knightville Dam 1941	37,900 Sept. 21, 1938	6,660 March 21, 1945	323	335 (through Sept. 1984)	
Slyke Brook at Knightville	1.73	June 1946–July 1974	—	680 August 19, 1955	—	258	—	
Middle Branch Westfield River at Cinn Heights	52.7	July 1910–September 1986	Littleville Dam 1965	19,900 Sept. 21, 1938	1,670 March 26, 1980	103	112 (through Sept. 1984)	
Walker Brook near Becket Center	2.94	November 1962–September 1977	—	499 August 10, 1976	—	6.67	—	
West Branch Westfield River near Huntington	94.0	September 1935–September 1986	—	26,100 August 19, 1955	—	192 (through Sept. 1984)	—	
Hubbard River near Westfield	22.6	November 1972–September 1982	—	1,350 June 6, 1982	—	35.8	—	
Westfield River near Westfield	497	June 1914–September 1986	Cobble Mountain–Borden Brook Dam, 1931; Knightville Dam, and Littleville Dam	42,500 November 6, 1927	70,200 August 19, 1955	885	942 (through Sept. 1984)	
Fall River below Otis Res., near Otis	16.5	August 1969–September 1982	Otis Dam 1865	—	422 July 2, 1977	—	37.7	—
West Branch Farmington River near New Boston	91.7	May 1913–September 1986	Otis Dam	—	34,300 August 19, 1955	—	184 (through Sept. 1984)	—
Hubbard River near Westfield, Connecticut	20.8	January 1938–September 1955, September 1956–September 1986	—	10,500 August 19, 1955	—	39.9 (through Sept. 1984)	—	—
Valley Brook near Westfield, Connecticut	7.39	February 1940–September 1972	—	5,800 August 19, 1955	—	14.1	—	—

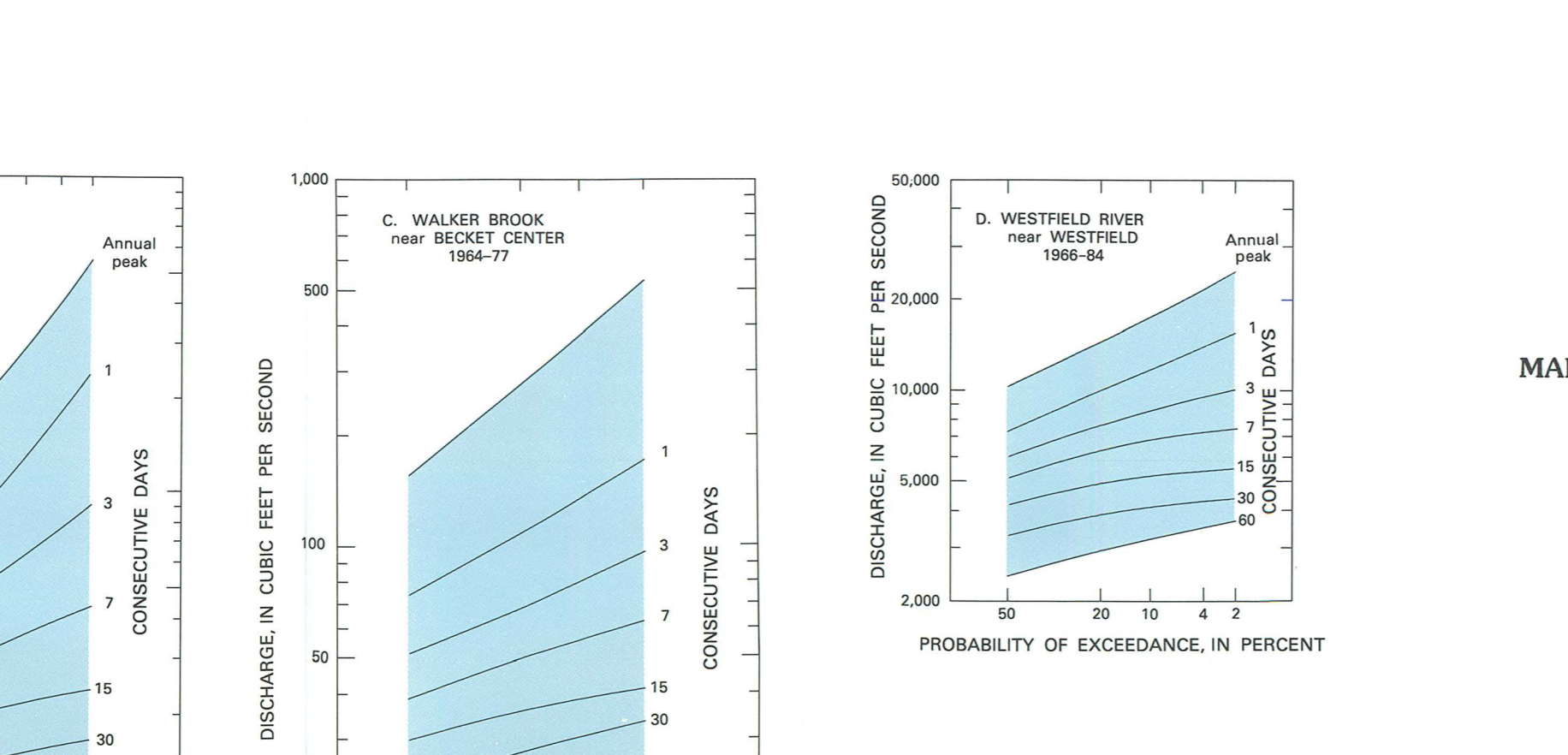
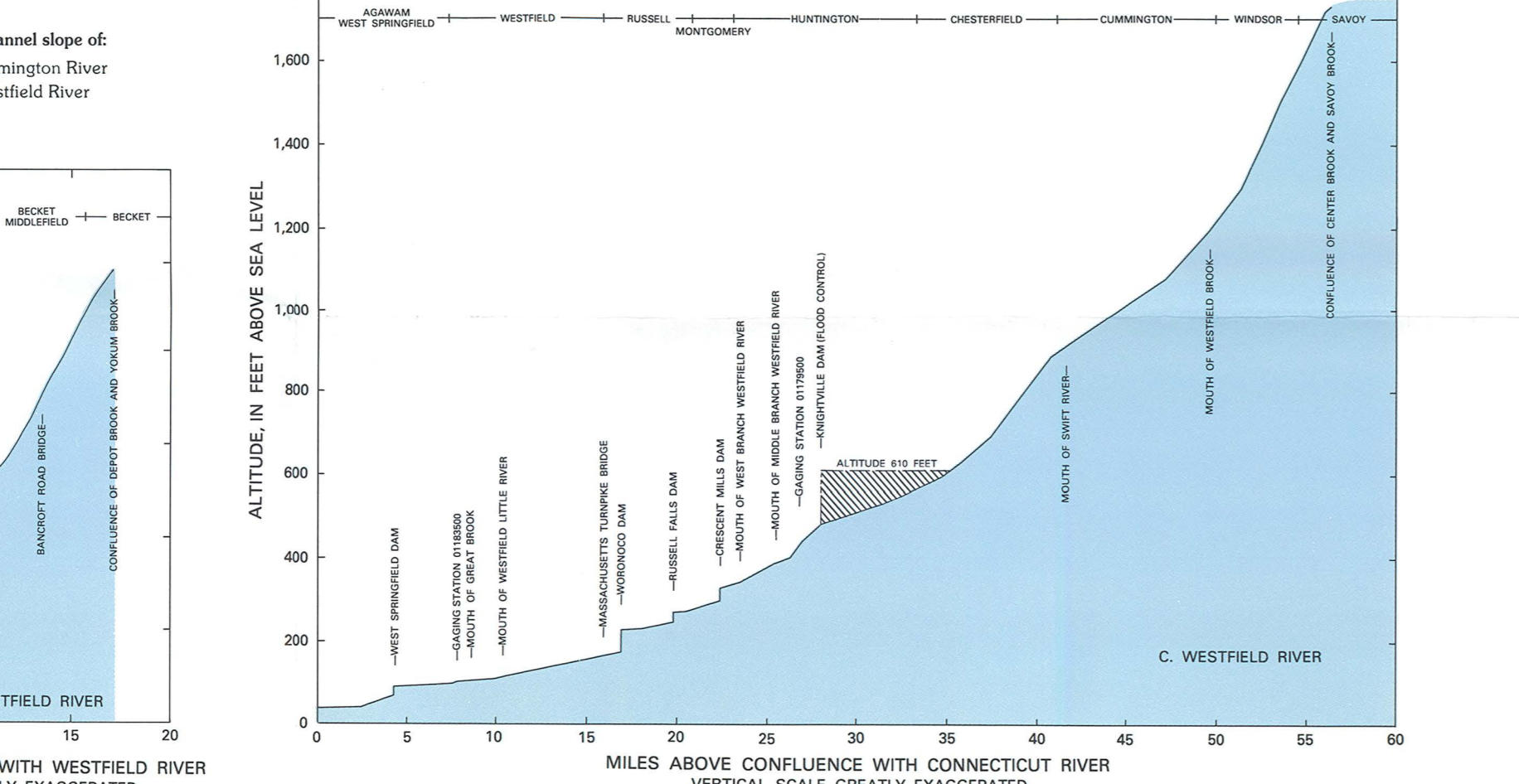
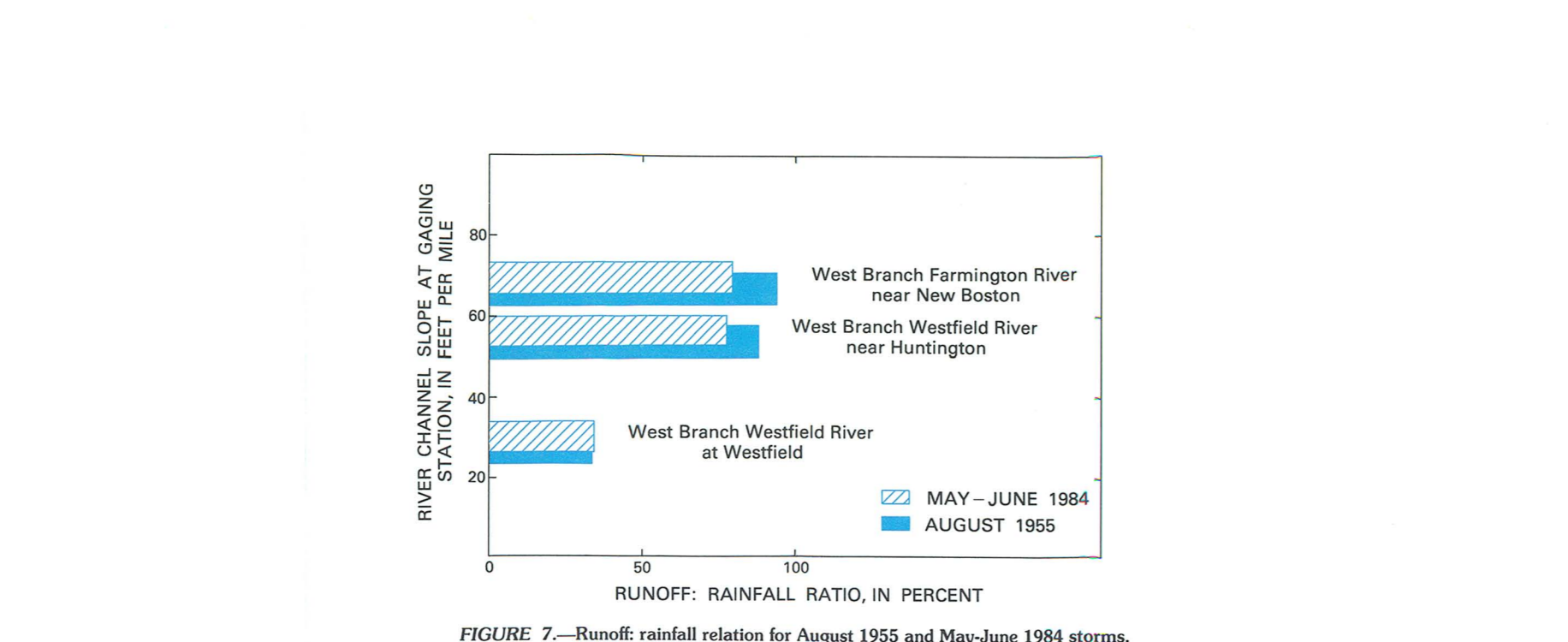


FIGURE 8.—Annual peak and flood-volume frequency curves for:  
A. West Branch Westfield River at Huntington  
B. Slyke Brook at Knightville  
C. Walker Brook near Becket Center  
D. Westfield River near Westfield  
E. West Branch Farmington River near New Boston  
F. Hubbard River near Westfield, Connecticut  
G. Valley Brook near Westfield, Connecticut

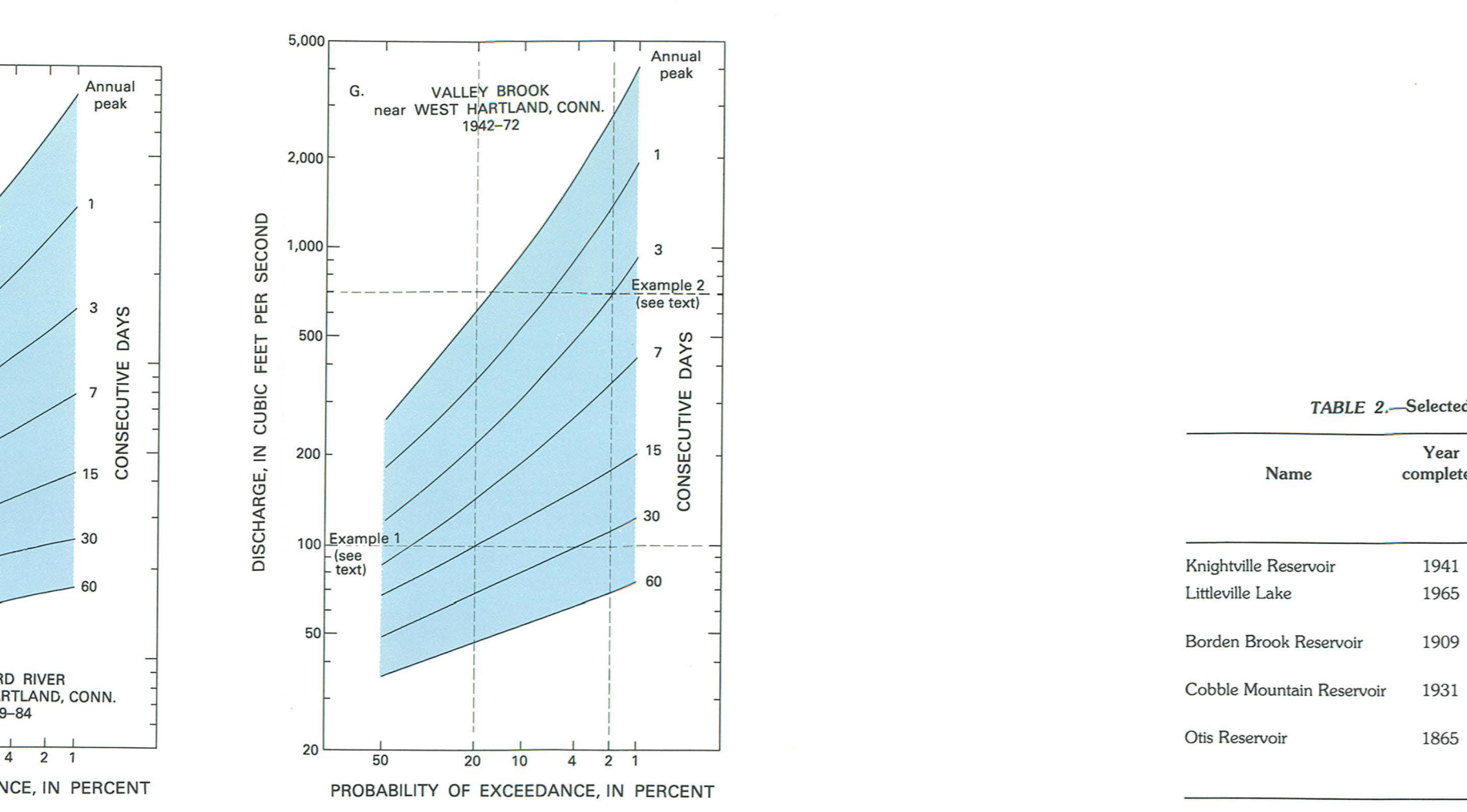


FIGURE 9.—Low-flow frequency curves for:  
A. West Branch Westfield River at Huntington  
B. Slyke Brook at Knightville  
C. Walker Brook near Becket Center  
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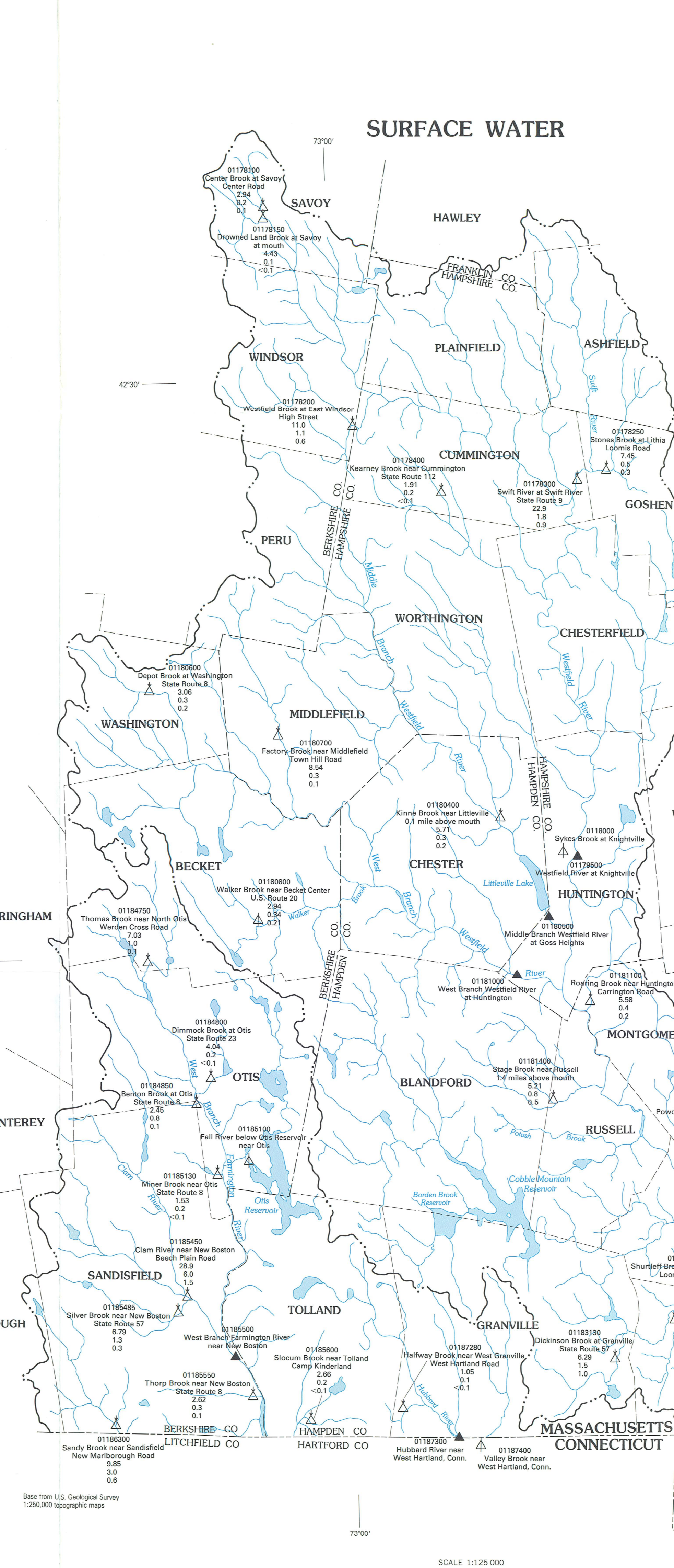


TABLE 2.—Selected manmade lakes and reservoirs

Name	Year completed	Usable capacity, in million gallons	Purpose/use
Knightville Reservoir	1941	15,900	Flood control
Littleville Lake	1965	10,500	Flood control, water supply, recreation
Borden Brook Reservoir	1969	2,570	Water supply
Cobble Mountain Reservoir	1931	22,800	Hydroelectric power
Otis Reservoir	1865	5,830	Hydroelectric power, recreation

**DIVERSIONS**

The Borden Brook and Cobble Mountain Reservoirs located west of the City of Westfield are the second largest water-supply storage systems in the State and provide water for the Springfield area southeast of the study area. Together, the reservoirs have a combined storage capacity of 25,400 Mgal (million gallons). Water is diverted underground by way of Wilcox and Springfield Aquifers for public supply to the City of Springfield and also for Worcester Air Force Base and the towns of Agawam, East Longmeadow, Longmeadow, Southwick, and West Springfield, all located about the northeastern corner of the study area. During 1985, an average of 17.2 Mgal was diverted from the Westfield River basin through the Springfield water system.

The Littleville Lake is located in the town of Chester and is used as an emergency supply for the Springfield water system. The storage capacity of the reservoir is 14,265 Mgal.

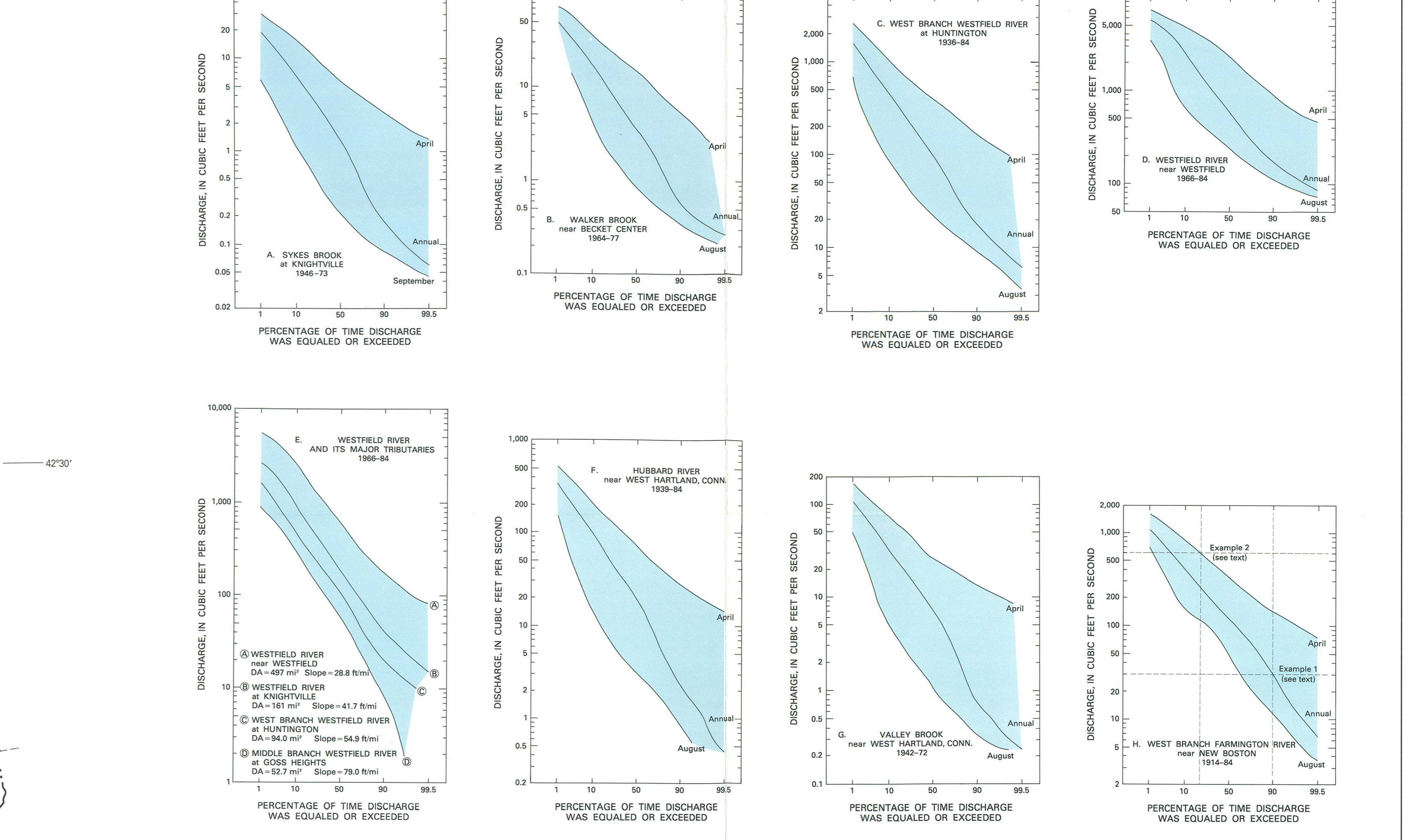


FIGURE 10.—Flow-duration curves for:  
A. Slyke Brook at Knightville  
B. Walker Brook near Becket Center  
C. West Branch Westfield River at Huntington  
D. Westfield River near Westfield  
E. Hubbard River near Westfield, Connecticut  
F. Valley Brook near Westfield, Connecticut  
G. West Branch Farmington River near New Boston

**FLOW DURATION**

Flow-duration curves for the streamflow-gaging stations in the Westfield and Farmington River basins (fig. 10) show the percentage of time a particular discharge was equaled or exceeded. The climatic, geologic, and hydrologic characteristics of a basin determine the shape of a duration curve. These characteristics include precipitation, evaporation, evapotranspiration, topography, channel slope, ground-water storage, and regulation of flow. The curves for Slyke Brook at Knightville, Walker Brook near Becket Center, West Branch Westfield River at Huntington, Westfield River near Westfield, Valley Brook near Westfield, Connecticut, Hubbard River near Westfield, Connecticut, and West Branch Farmington River near New Boston also show duration curves of the daily mean flow for the months that had the highest and lowest median flows.

The shape of duration curves for the Westfield River and some of its major tributaries (fig. 10E) shows the relations between drainage area, channel slope, and streamflow. Reaches along the river with larger drainage area and less channel slope about the gaging station permit higher sustained streamflow discharge with smaller drainage areas and more channel slope about the gaging station. The annual period used in the duration curves is the water year—the 12-month period beginning October 1 and ending September 30 of the year designated.

The following two examples refer to the flow-duration curve for the West Branch Farmington River near New Boston (fig. 10G).

Example No. 1: A flow of 30 ft<sup>3</sup>/s is equaled or exceeded 90 percent of the time throughout the year.

Example No. 2: The flow equals or exceeds 600 ft<sup>3</sup>/s 20 percent of the time during the month of April.

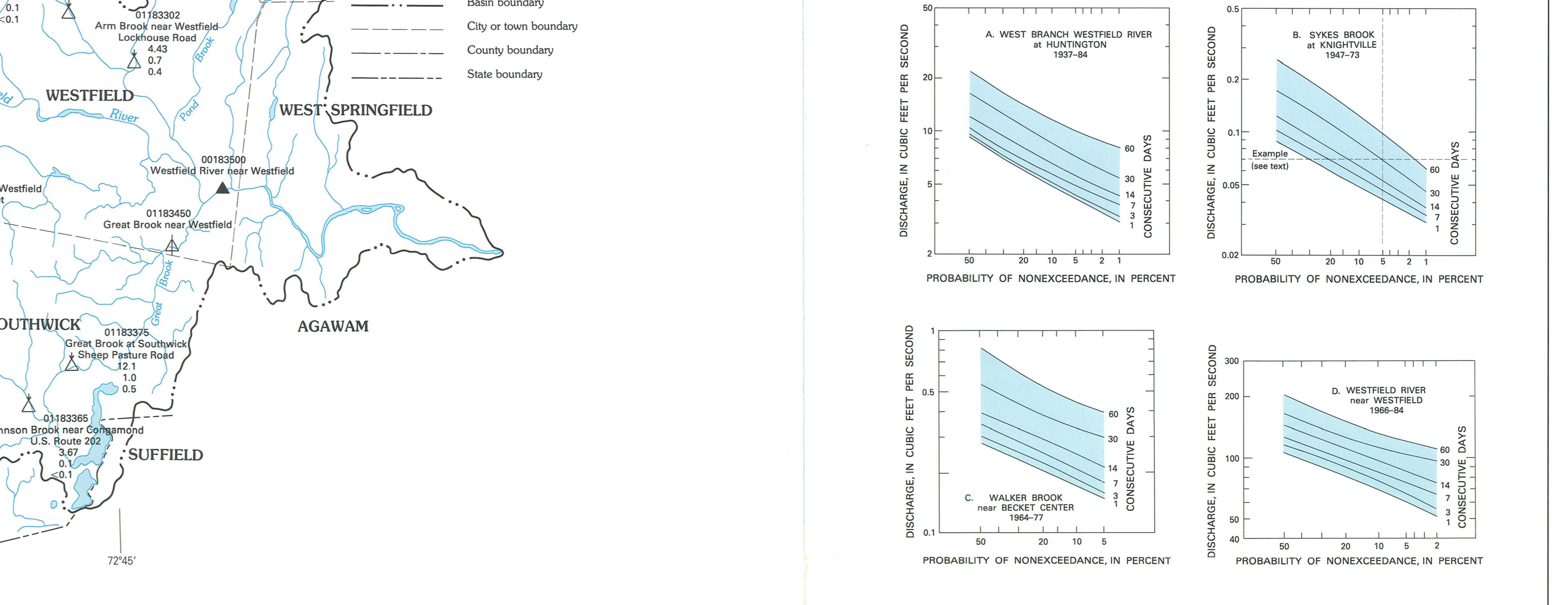


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**WATER RESOURCES OF THE WESTFIELD AND FARMINGTON RIVER BASINS, MASSACHUSETTS**

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